

## 3.7 WATER RESOURCES

This section discusses water resources in the North I-25 regional study area.

Numerous streams, tributaries, canals, ditches, reservoirs, and lakes in the regional study area watersheds are either adjacent to or cross I-25 and the other major corridors (US 85, Union Pacific Railroad (UPRR), and Burlington Northern Santa Fe Railway (BNSF) in the project area.

Surface waters contribute to the quality of life for residents within the regional study area because they provide water supply, recreation opportunities, and aesthetic value. The aquatic and riparian habitats that the surface waters provide are vital for a wide variety of species within the project area. Increased urbanization and mixed land use practices within the regional study area and project area are progressively contributing to degraded water quality. Accordingly, protecting the integrity of water resources within the project area is a critical piece of this project, which is legally mandated by federal, state, and local regulations.

This section provides an overview of the existing conditions of surface water in the project area and assesses impacts that the build packages (Package A, Package B, and the Preferred Alternative) and the No-Action Alternative would have on water quality due to project activities. Impacts to groundwater wells are also addressed in this section. Permanent best management practices (BMPs) have been incorporated into the roadway and rail design for both packages to ensure MS4 compliance and reduce the majority of impacts from stormwater. Consequently, it is anticipated that water quality conditions will improve when compared to the existing conditions in areas where no water quality treatment is currently provided.

### 3.7.1 Water Resources Regulations

Water resources within the regional study area are managed through federal, state, and local regulations that establish the standards and management actions necessary to protect their physical, chemical, and biological integrity. The primary regulations governing surface water and groundwater resources in the project area are the Clean Water Act (CWA) and Safe Drinking Water Act (SDWA). The Colorado Department of Public Health and Environment (CDPHE) Water Quality Control Commission (WQCC) has the authority to establish and enforce water quality standards within the state.

The primary water quality concern associated with the project results from the discharge of stormwater to receiving waters (see **Section 3.7.3**). As part of the CWA, entities with stormwater discharges are regulated under the National Pollutant Discharge Elimination System (NPDES) permit program.

#### What's in Section 3.7?

##### 3.7 Water Resources

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1 Municipal Separate Storm Sewer Systems (MS4) that are owned and maintained by  
2 municipalities and CDOT are required to obtain Colorado Discharge Permit System (CDPS)  
3 permits for stormwater discharges. The permit requires the Colorado Department of  
4 Transportation (CDOT) to develop and implement a stormwater management program to  
5 maintain and protect water quality conditions from their stormwater discharges. A major  
6 program element is the development and implementation of BMPs, which are defined as  
7 activities, procedures, and other practices that prevent or reduce water pollution. As part of the  
8 MS4 program, CDOT is required to design, construct, and maintain permanent BMPs to  
9 protect aquatic resources. As part of the stormwater management program, CDOT also is  
10 required to develop, implement, and enforce a program to reduce pollutants in stormwater  
11 runoff for any construction activity that would result in a land disturbance greater than or equal  
12 to one acre.

13 While the entire project must comply with CDPHE-WQCC rules and regulations (including  
14 construction requirements), the MS4 permit requirements are only applicable in designated  
15 MS4 areas. Because of the size of this project, the build packages cross 11 MS4 areas  
16 (including municipalities and portions of counties). The CDOT MS4 requirements described  
17 above are generally only applicable in these MS4 areas. Besides compliance with the CDOT  
18 MS4 program, all local MS4 construction and new development requirements must also be  
19 met within the local MS4 jurisdictional boundaries. An analysis was conducted using the 2000  
20 census data to define the permit coverage for portions of Adams, Larimer, and Boulder  
21 counties based on population density. Because the regional study area is rapidly growing, the  
22 projected 2035 population used in the traffic model was utilized to predict what areas might be  
23 within an MS4 area in 2035. The project should also comply with additional requirements of  
24 local municipal MS4 programs. The final coordination of these permit issues is typically  
25 completed during the design phase of the project. The CDOT MS4 requirements and  
26 specifications comply with the FHWA regulation "Erosion and Sediment Control on Highway  
27 Construction Projects". More detailed information on CDOT MS4 permit requirements are  
28 provided in the *Water Quality and Floodplains Technical Report* (FHU, 2008c) and *Addendum*  
29 *(FHU, 2010b)*.

### 30 **3.7.1.1 SURFACE WATER CLASSIFICATIONS**

31 Two main regulations have been established by the CDPHE-WQCC that classify the  
32 designated uses and water quality standards that apply to the surface water bodies within the  
33 project area.

- 34 ▶ Regulation 31 - Basic Standards and Methodologies for Surface Water
- 35 ▶ Regulation 38 - Classification and Numeric Standards for South Platte River Basin;  
36 Laramie River Basin; Republican River Basin; Smoky Hill River Basin

37 Colorado has four designated uses for surface water bodies: agriculture, water supply,  
38 recreation, and aquatic life. These designated uses have their own unique water quality  
39 standards that are either numeric (quantitative thresholds) or narrative (visual/aesthetic).  
40 Surface water classifications do not apply to water that is conveyed in man-made structures  
41 such as ditches. Streams that do not meet established water quality standards ("impaired  
42 streams") are placed on the Colorado 303(d) List and are required to go through a process to  
43 help improve water quality. The process results in the development of a Total Maximum Daily  
44 Load (TMDL), which is a total amount of pollutant loading that a surface water system can

1 assimilate without exceeding water quality standards. Surface waters that require additional  
2 monitoring and evaluation to determine if water quality standards are being met are placed on  
3 the Colorado 303(d) Monitoring and Evaluation List.

4 The watersheds within the project area contain numerous canals and ditches that transport  
5 water for irrigation and domestic drinking water supply. However, canals and ditches do not  
6 have designated uses as do natural watercourses. According to State of Colorado code  
7 (C.R.S. § 25-8-203(2)(f)), "Waters in ditches and other man-made conveyance structures shall  
8 not be classified with designated uses, and water quality standards shall not be applied to  
9 them but may be utilized for purposes of discharge permits" [CDPHE-WQCD, 2003].

10 The designated uses for the surface water bodies within the project area and impaired  
11 segments are listed in **Table 3.7-1**. Impaired stream segments are included in **Figure 3.7-3**.  
12 Stream segments on the Monitoring and Evaluation List for potential highway-related  
13 constituents are included in **Table 3.7-1** and **Figure 3.7-3**. The TMDL status for impaired  
14 streams is included in the *Water Quality and Floodplains Technical Report* (FHU, 2008c) and  
15 *Addendum* (FHU, 2011b).

### 16 **3.7.1.2 SAFE DRINKING WATER ACT**

17 Public drinking water supplies (systems serving more than 25 people) from both groundwater  
18 and surface water sources are regulated by the Safe Drinking Water Act (SDWA). These  
19 sources include lakes, rivers, reservoirs, springs, and groundwater. Under the SDWA,  
20 Environmental Protection Agency (EPA) and the Congress established national health-based  
21 standards for drinking water contaminants specified as having known adverse human health  
22 effects. As with the CWA, EPA has delegated regulatory authority of the Safe Drinking Water  
23 Act to the CDPHE Water Quality Control Division (WQCD). **Section 3.7.3** includes information  
24 about public water supply wells in the project area.

### 25 **3.7.1.3 SENATE BILL 40**

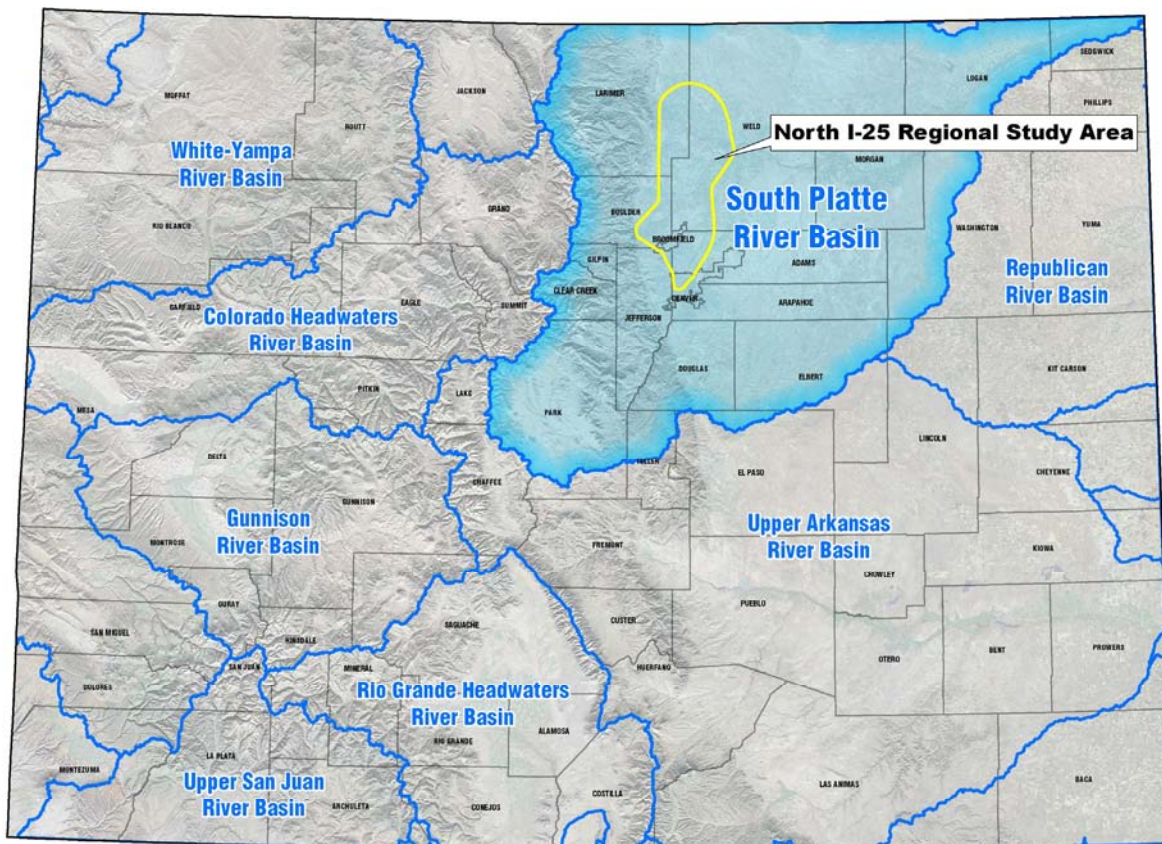
26 Colorado Senate Bill 40 (SB40) requires that projects that affect waters of the state and their  
27 associated riparian areas comply with its provisions. These provisions are aimed at preserving  
28 wildlife habitat in streams for fish and aquatic species and terrestrial species that rely upon  
29 riparian areas. Compliance with SB40 provisions is documented in a certification obtained  
30 through the Colorado Division of Wildlife (CDOW). **Section 3.7.4** includes information about  
31 SB40 guidelines that will be followed in the project area.

## 32 **3.7.2 Affected Environment**

### 33 **3.7.2.1 SURFACE WATER**

34 The regional study area lies in the transition zone between the Rocky Mountain Front Range in  
35 central Colorado and the Great Plains of eastern Colorado and is situated entirely in the South  
36 Platte River basin (see **Figure 3.7-1**). The South Platte River basin, which is one of eight  
37 major river basins in Colorado, occupies approximately 13 million acres in Colorado, Wyoming,  
38 and Nebraska.

1 **Figure 3.7-1 South Platte River Basin**



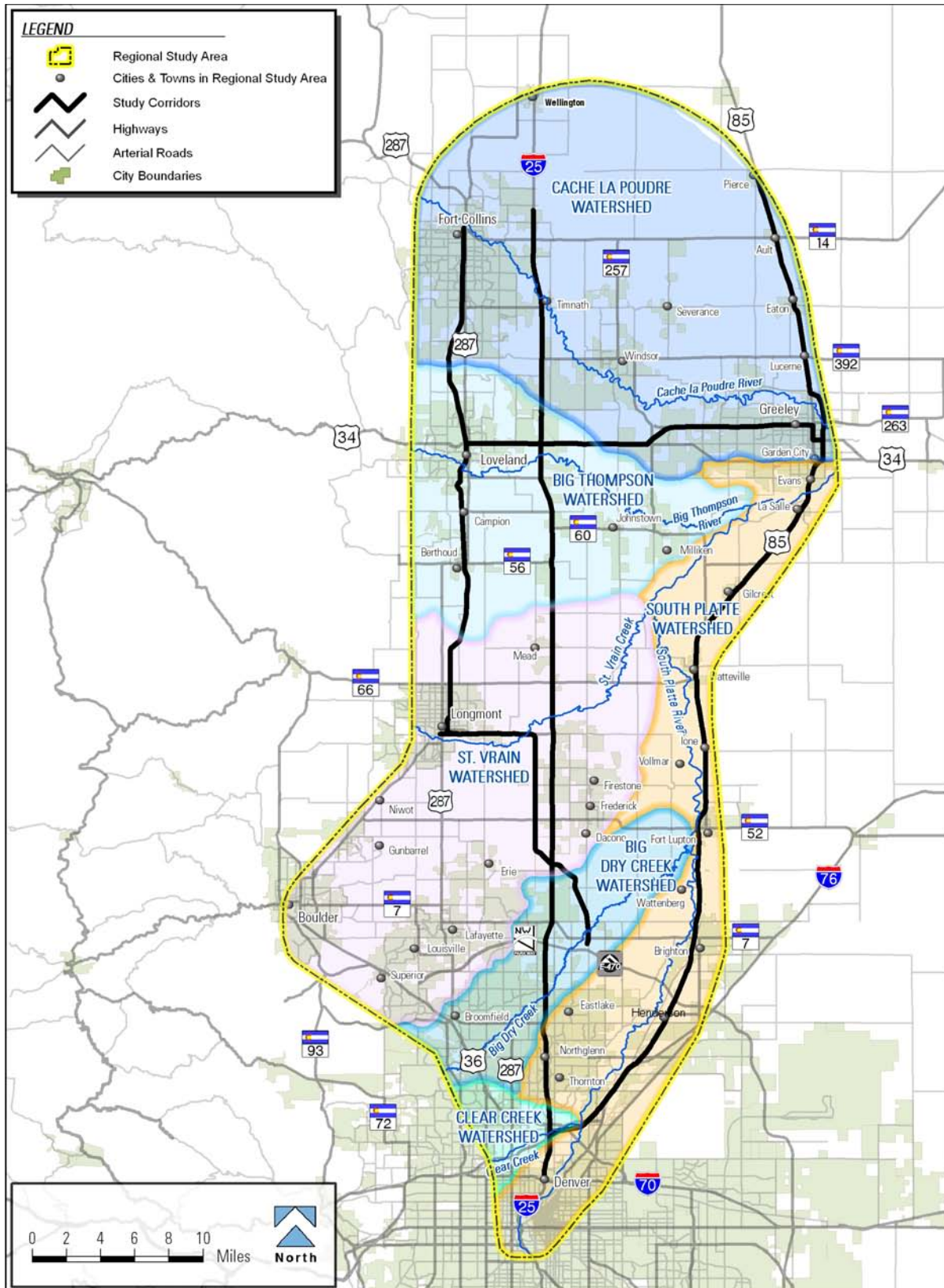
2 Six main watersheds occur in the regional study area: the South Platte River, Clear Creek, Big  
3 Dry Creek, St. Vrain Creek/Boulder Creek, Big Thompson River, and Cache la Poudre River.  
4 Numerous streams, tributaries, canals, ditches, reservoirs, and lakes in these watersheds are  
5 either adjacent to or cross I-25 and the other major corridors (US 85, UPRR, and BNSF  
6 Railway) in the project area (see **Figure 3.7-2**).

7 Hydrology and stream flow regime characteristics of the six watersheds in the regional study  
8 area are very similar. The majority of stream flow originates as snowmelt, creating high-flow  
9 conditions from May to July, with peak flows in June, and lower flows from October to March.  
10 Natural hydrologic conditions in the basin's watersheds have been altered because of  
11 extensive in-basin and trans-basin water diversions, reservoir construction, and discharges  
12 from publicly-owned treatment works (POTW) (USGS, 1998).

13 Numerous man-made surface water drainage features are also present within the project area  
14 and include culverts, inlets, and open channels. Most of the existing drainage structures in the  
15 project area were built during the 1960s. At that time, the adjacent areas were rural, and flood  
16 damage was limited to agricultural land. The sizes of many of these drainage structures were  
17 based on limited rainfall data for what was estimated to be a 25- or 50-year storm event. The  
18 100-year storm is now used for drainage design in urbanized areas and for floodplains under  
19 the jurisdiction of the Federal Emergency Management Agency (FEMA). Many of the existing  
20 drainage structures constrict stormwater flows, cause flooding, and overtopping of the adjacent  
21 highways. In order to conform to newer criteria and control flooding, most drainage structures  
22 in the project area will be replaced with larger structures.

1 Figure 3.7-2 Watersheds in the Regional Study Area

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1 **3.7.2.2 WATERSHEDS**

2 This section describes the surface water designated uses and water quality impairments within  
3 the project area. Stream segments on CDPHE's Monitoring and Evaluation List for potential  
4 highway-related constituents are also included in this section. Water bodies that cross or are  
5 present within 100 feet of the existing I-25 or US 85 edges-of-pavement or the edge of the rail  
6 lines were considered to be within the project area. However, in certain cases, water bodies  
7 outside the project area were also included if they are: 1) downstream from the project area,  
8 2) designated water supplies, or 3) impaired and close to the project area.

9 Existing contaminant loading from the current highway configuration for each watershed was  
10 estimated using an FHWA water quality model (Driscoll Model). This model is discussed later  
11 in this section. Five contaminants were modeled for the project area (chloride, copper,  
12 phosphorus, total suspended solids (TSS), and zinc) because of their water quality  
13 implications in the project area. They are assumed to be an indicator of overall contamination  
14 in runoff.

15 *South Platte River Watershed*

16 The South Platte River watershed occupies 45,560 acres in the southern portion of the  
17 regional study area (see **Figure 3.7-2**). Overall, within this watershed, I-25 accounts for  
18 approximately 110 acres of impervious surface within the project area (USGS, 2000). The  
19 E-470 to US 36 component crosses this watershed.

20 The stream segments within the project area, their designated stream uses, and any  
21 impairments are listed in **Table 3.7-1**. The main stem (Segments 15, 1a, and 1b) is also  
22 included because it has water supply designations and all streams within the project area  
23 eventually discharge into the South Platte River. It is important to consider downstream  
24 segments to ensure that upstream project activities do not adversely affect those receiving  
25 water bodies. Segment 15 is the only segment with water quality impairments. This portion of  
26 the main stem has been placed on the 2006 303(d) List for an *E. coli* impairment (see  
27 **Figure 3.7-3**) (CDPHE-WQCD, 2006a). *E. coli* is not generally associated with roadway runoff.

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**Table 3.7-1 Surface Water Segments, Designated Uses, and Impairments within the Project Area**

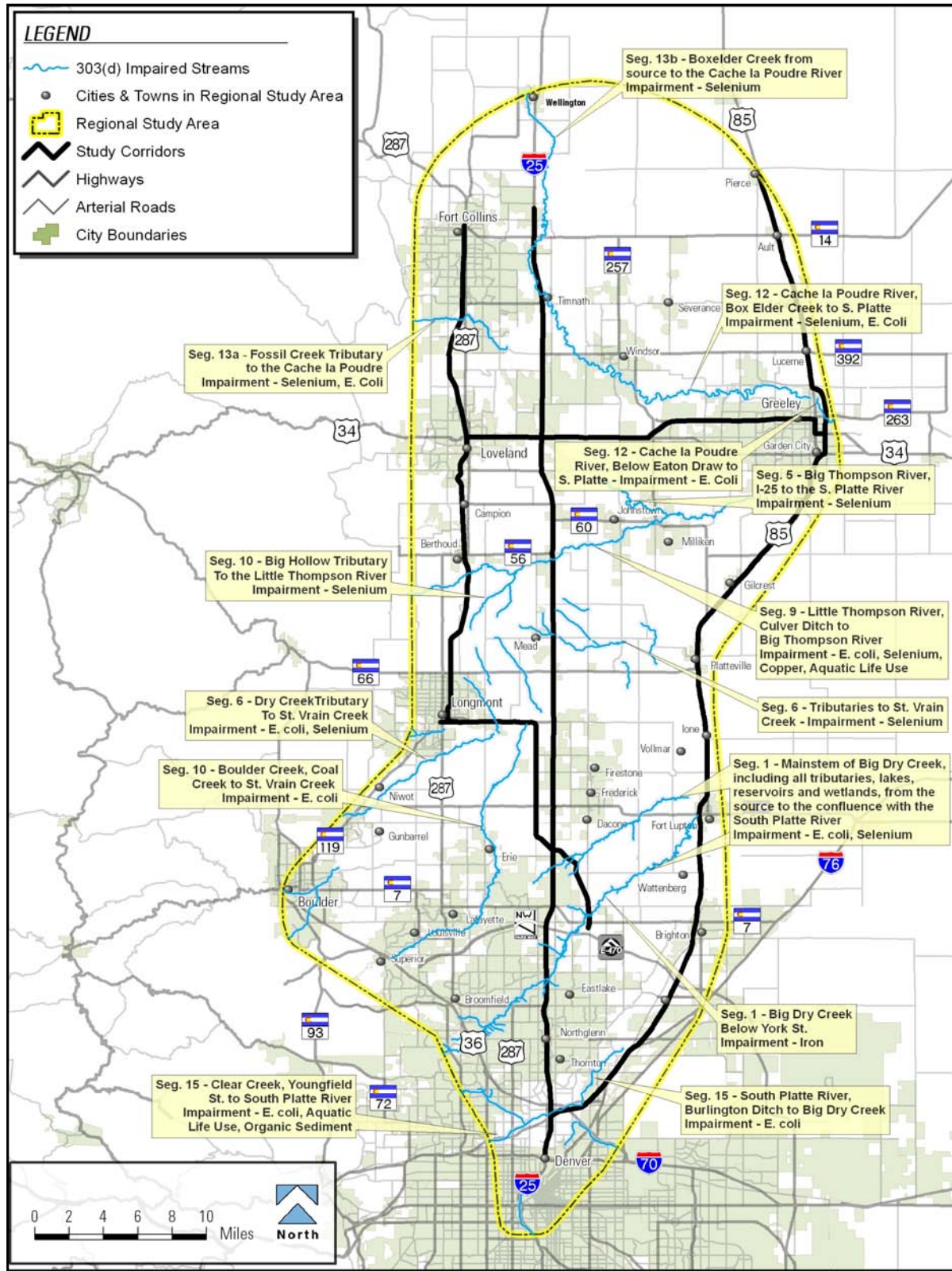
	<b>Designated Uses</b>	<b>Impairments</b>
<b>South Platte River Watershed</b>		
Segment 15	Recreation E, Class 2 warm water aquatic life, agriculture, and water supply	<i>E. coli</i>
Segment 16c	Recreation E, Class 2 warm water aquatic life, and agriculture	N/A
Segment 1a	Recreation E, Class 2 warm water aquatic life, agriculture, and water supply	N/A
Segment 1b	Recreation E, Class 2 warm water aquatic life, agriculture, and water supply	N/A
<b>Clear Creek Watershed</b>		
Segment 15	Class 1 warm water aquatic life, recreation E, agriculture, and water supply	<i>E. coli</i> , aquatic life use, and organic sediment
<b>Big Dry Creek Watershed</b>		
Segment 1	Class 2 warm water aquatic life, recreation P, and agriculture	<i>E. coli</i> and Selenium; Total Recoverable Iron (M & E List)
<b>St. Vrain/Boulder Creek Watershed</b>		
Segment 3	Class 1 warm water aquatic life, recreation E, and agriculture	<i>E. coli</i> , aquatic life use
Segment 6	Class 2 warm water aquatic life, recreation E, and agriculture	<i>E. coli</i> (Dry Creek Only), Selenium
Segment 10	Class 1 warm water aquatic life, recreation E, agriculture, and water supply.	<i>E. Coli</i>
<b>Big Thompson River Watershed</b>		
Segment 4b	Class 2 warm water aquatic life, recreation E (from 5/1 to 10/15 annually), and recreation 2 (10/16 to 4/30, annually)	N/A
Segment 4c	Class 2 warm water aquatic life, recreation E (from 5/1 to 10/15 annually), and recreation 2 (10/16 to 4/30, annually)	N/A
Segment 5	Class 2 warm water aquatic life, recreation P (from 5/1 to 10/15, annually), recreation N (10/16 to 4/30, annually), and agriculture	Selenium
Segment 6	Class 2 warm water aquatic life, recreation E, and agriculture	N/A
Segment 9	Class 2 warm water aquatic life, recreation N, and agriculture	Selenium, <i>E. coli</i> , Copper, Aquatic life
Segment 10	Class 2 warm water aquatic life, recreation N, and agriculture	Selenium (Big Hollow)
<b>Cache la Poudre River Watershed</b>		
Segment 11	Class 2 warm water aquatic life, recreation E, and agricultural uses	N/A
Segment 12	Class 2 warm water aquatic life, recreation E, and agricultural uses	<i>E. coli</i> (below Eaton Draw), Selenium
Segment 13a	Class 2 warm water aquatic life, recreation E, and agricultural uses	Selenium, <i>E. coli</i>
Segment 13b	Class 2 warm water aquatic life, recreation P (5/15 to 9/15, annually), recreation N (9/16 to 5/14, annually), and agricultural uses	Selenium

Source: CDPHE, 2007 original

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1 Figure 3.7-3 Impaired Streams in the Regional Study Area

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1 **Table 3.7-2** presents the estimated existing contaminant loading from a storm event from the  
2 existing I-25 conditions in the South Platte River watershed. These values are compared to the  
3 estimated loading for each alternative in the following section.

4 **Table 3.7-2 Mean Contaminant Loading Per Storm Event From The Driscoll Model**  
5 **(Pounds per Event) in the South Platte River Watershed**

Watershed	Chloride (pounds/event)	Copper (pounds/event)	Phosphorus (pounds/event)	Total Suspended Solids (TSS) (pounds/event)	Zinc (pounds/event)
South Platte River	78.4	0.058	3.7	2,600	0.52
Clear Creek	14.5	0.011	0.68	481	0.097
Big Dry Creek	125	0.093	5.8	4,150	0.83
St. Vrain Creek	265	0.20	12.4	8,800	1.8
Big Thompson River	181	0.13	8.4	6,000	1.2
Cache la Poudre River	266	0.20	12.4	8,800	1.8

6 *Clear Creek Watershed*

7 The Clear Creek watershed occupies 14,787 acres in the southern portion of the regional  
8 study area (see **Figure 3.7-2**). Overall, within this watershed, I-25 accounts for approximately  
9 20 acres of impervious surface within the project area (USGS, 2000). The E-470 to US 36  
10 (H4) component crosses this watershed.

11 The stream segments within the project area, their designated stream uses, and any  
12 impairments are listed in **Table 3.7-1**. Clear Creek Segment 15 is located downstream of the  
13 project area and has been placed on the 2006 303(d) List for *E. coli*, aquatic life use, and  
14 organic sediment (CDPHE-WQCD, 2006a). Constituents causing the stream impairments near  
15 the project area are generally not associated with roadway runoff.

16 **Table 3.7-2** presents the estimated existing contaminant loading from a storm event for the  
17 existing I-25 conditions in the Clear Creek watershed. These values are compared to the  
18 estimated loading for each alternative in the following section.

19 *Big Dry Creek Watershed*

20 The Big Dry Creek watershed occupies 65,055 acres in the southern portion of the regional  
21 study area. The watershed lies south of the St. Vrain Creek watershed and north of the South  
22 Platte River watershed (see **Figure 3.7-2**). Overall, within this watershed, I-25 accounts for  
23 171 acres of impervious surface area within the project area (USGS, 2000). The E-470 to  
24 US 36 component and the SH 60 to E-470 component cross this watershed.

25

1 The stream segments within the project area, their designated stream uses, and any  
2 impairments are listed in **Table 3.7-1**. Big Dry Creek (Segment 1) is within the project area and  
3 has been placed on the 2006 303(d) List for *E. coli* and selenium (CDPHE-WQCD, 2006a).  
4 *E. coli* and selenium are generally not associated with roadway runoff.

5 A portion of Segment 1, located approximately 2.5 miles downstream of the project area, has  
6 also been placed on the 2006 Monitoring and Evaluation List for total recoverable iron  
7 (CDPHE-WQCD, 2006b). Iron is a constituent that can be associated with roadway runoff due  
8 to auto body rust, steel highway structures, and vehicle engine parts.

9 **Table 3.7-2** presents the estimated existing contaminant loading from a storm event for the  
10 existing I-25 conditions in the Big Dry Creek watershed. These values are compared to the  
11 estimated loading for each alternative in the following section.

### 12 *St. Vrain Creek Watershed*

13 The St. Vrain Creek watershed occupies 204,664 acres in the middle portion of the regional  
14 study area. The watershed lies north of the Big Dry Creek watershed and south of the Big  
15 Thompson River watershed (see **Figure 3.7-2**). Overall, within this watershed, I-25 accounts  
16 for 350 acres of impervious surface area within the project area (USGS, 2000). The SH 60 to  
17 E-470 component crosses this watershed.

18 The stream segments within the project area, their designated stream uses, and any  
19 impairments are listed in **Table 3.7-1**. Segment 3 has been placed on the 2006 303(d) List for  
20 *E. coli* (CDPHE-WQCD, 2006a). Segment 6 has been placed on the 2006 303(d) List for  
21 selenium and *E. coli* (CDPHE-WQCD, 2006a). Boulder Creek (Segment 10) is also included  
22 because it is located close to the project area, has a designated water supply designation, and  
23 has an impairment for *E. coli* (CDPHE-WQCD, 2006a). *E. coli* and selenium are generally not  
24 associated with roadway runoff.

25 **Table 3.7-2** presents the estimated existing contaminant loading from a storm event for the  
26 existing I-25 conditions in the St. Vrain Creek watershed. These values are compared to the  
27 estimated loading for each alternative in the following section.

### 28 *Big Thompson River Watershed*

29 The Big Thompson watershed occupies 122,523 acres in the northern portion of the regional  
30 study area (see **Figure 3.7-2**). The watershed is located north of the St. Vrain Creek  
31 watershed and south of the Cache la Poudre River watershed. Overall, within this watershed,  
32 I-25 accounts for approximately 223 acres of impervious surfaces within the project area  
33 (USGS, 2000). The SH 14 to SH 60 component and SH 60 to E-470 component cross this  
34 watershed.

35 The stream segments within the project area, their designated stream uses, and any  
36 impairments are listed in **Table 3.7-1**. Segments 5, 9, and 10 have been placed on the 2006  
37 303(d) List for selenium (all segments), ammonia (Segment 5), and *E. coli* (Segment 9)  
38 (CDPHE-WQCD, 2006a). Ammonia, *E. coli*, and selenium are generally not associated with  
39 roadway runoff.

40 **Table 3.7-2** presents the estimated existing contaminant loading from a storm event for the  
41 existing I-25 conditions in the Big Thompson River watershed. These values are compared to  
42 the estimated loading for each alternative in the following section.

1 *Cache la Poudre River Watershed*

2 The Cache la Poudre River watershed occupies 264,736 acres in the northern portion of the  
3 project area. The watershed lies north of Big Thompson River watershed (see **Figure 3.7-2**).  
4 Overall, within this watershed, I-25 accounts for approximately 337 acres of impervious  
5 surfaces within the project area (USGS, 2000). The SH 1 to SH 14 component and SH 14 to  
6 SH 60 component cross this watershed.

7 The stream segments within the project area, their designated stream uses, and any  
8 impairments are listed in **Table 3.7-1**. Segments 12, 13a, and 13b have been placed on the  
9 2006 303(d) List for selenium. Segment 12 has also been placed on the 2006 303(d) List for  
10 *E. coli* (CDPHE-WQCD, 2006a). *E. coli* and selenium are generally not associated with  
11 roadway runoff.

12 **Table 3.7-2** presents the estimated existing contaminant loading from a storm event for the  
13 existing I-25 conditions in the Cache la Poudre River watershed. These values are compared  
14 to the estimated loading for each alternative in the following section.

15 **3.7.2.3 GROUNDWATER**

16 Numerous groundwater wells are located within the regional study area. The regional study  
17 area is situated above the consolidated bedrock aquifers of the Denver basin and Dakota-  
18 Cheyenne group (aka South Platte Formation) and the unconsolidated shallow alluvial aquifers  
19 associated with the South Platte River and its tributaries (Colorado Geological Survey, 2003).  
20 Groundwater from the aquifers can be brought to the surface with wells and provide water  
21 supply for multiple uses. The Denver basin aquifers primarily supply domestic and municipal  
22 water. The Dakota-Cheyenne group primarily supplies domestic, livestock, and industrial  
23 water. The South Platte Valley-Fill alluvial aquifer primarily supplies irrigation and municipal  
24 water.

25 **3.7.3 Environmental Consequences**

26 This section describes the potential consequences of the No-Action Alternative, Package A,  
27 Package B, and the Preferred Alternative with regard to water quality and stormwater drainage  
28 for the six watersheds within the project area. Permanent BMPs, consisting of water quality  
29 ponds, have been incorporated into the roadway and rail design for both packages to ensure  
30 MS4 compliance. During final design, BMPs other than water quality ponds may be used. It is  
31 anticipated that any alternate BMPs will be able to be incorporated within the right-of-way  
32 identified in this EIS for water quality ponds. Consequently, it is anticipated that water quality  
33 conditions will improve when compared to the existing conditions in areas where no water  
34 quality treatment is currently provided.

35 **3.7.3.1 WATER QUALITY IMPACTS METHODOLOGY**

36 *Surface Water*

37 If stormwater is left untreated, water quality impacts are generally correlated with the addition  
38 of paved impervious surfaces that alter the volume, velocity, and quality of stormwater runoff  
39 discharged into nearby surface water bodies. The impacts common to all alternatives that  
40 affect water quality in the absence of BMPs are listed in **Table 3.7-3**.

41

1 Table 3.7-3 Common Highway-Related Surface Water Quality Impacts

	Direct Impacts	Typical Mitigation <sup>1</sup>
<b>Sediment</b>	<b>Harmful to aquatic life.</b> Sedimentation directly degrades aquatic habitat. Suspended sediment increases turbidity and reduces aquatic plant life productivity. Suspended sediment can be fatal to aquatic species by reducing dissolved oxygen levels (Trombulak and Frissell, 2000).	<ul style="list-style-type: none"> <li>▶ <b>Water Quality Ponds<sup>2</sup></b></li> <li>▶ <b>Channel Stabilization</b></li> <li>▶ <b>Nonstructural BMPs</b> (continued decreasing use of salt and sanding)</li> </ul>
<b>Anti-Icing / De-Icing Chemicals (Salt-Based Deicers)</b>	<b>Potentially harmful to aquatic species, including plants.</b> CDOT is conducting research to better understand the aquatic life effects.	<ul style="list-style-type: none"> <li>▶ <b>Nonstructural BMPs</b> (continued decreasing use of salt and sanding)</li> </ul>
<b>Petroleum</b>	<b>Toxic to aquatic life.</b> Typically accumulates on the water surface and can inhibit plant and animal productivity. Direct mortality. The severity of the petroleum effects is related to the habitat into which it is introduced. River habitats may be less severely affected by spills than standing water.	<ul style="list-style-type: none"> <li>▶ <b>Water Quality Ponds<sup>2</sup></b></li> <li>▶ <b>Nonstructural BMPs</b> (Spill Prevention Plans &amp; Emergency Notification Procedures)</li> </ul>
<b>Metals</b>	<b>Toxic to aquatic life. Bio-accumulation.</b> Metals that bind to suspended solids and decaying organic matter can persist in the environment for long periods of time. <b>Contamination of drinking water supplies.</b>	<ul style="list-style-type: none"> <li>▶ <b>Water Quality Ponds<sup>2</sup></b></li> <li>▶ <b>Well Abandonment</b></li> <li>▶ <b>Nonstructural BMPs</b> (Spill prevention plan during construction)</li> </ul>
<b>Nutrients</b>	<b>Toxic to aquatic life.</b> Excessive nutrients, primarily nitrogen and phosphorus, can cause extreme algal growth, which is toxic to certain aquatic organisms. Algal blooms and die-off causes large swings in dissolved oxygen levels and in extreme cases fish kills. <b>Alters aesthetics. Can cause designated use impairments.</b>	<ul style="list-style-type: none"> <li>▶ <b>Water Quality Ponds<sup>2</sup></b></li> </ul>
<b>General Construction Activities</b>	<b>Erosion. Harmful to aquatic life.</b> Vegetation removal at construction sites increases stormwater runoff velocity and volume causing accelerated erosion. Riparian vegetation removal reduces stream bank stability, accelerates erosion, alters aquatic habitat and shading, and causes in-stream temperature changes. Construction vehicles deposit sediment onto surrounding roads, which is later mobilized during storm events.	<ul style="list-style-type: none"> <li>▶ <b>Construction BMPs<sup>3</sup></b> <ul style="list-style-type: none"> <li>• Minimize in-stream activities</li> <li>• Stormwater Management Plan (silt fence, inlet protection, containerization of wastes, etc.)</li> <li>• Revegetation and replacement of site, including riparian areas</li> <li>• Spill Prevention Plan</li> <li>• Construction Phasing</li> </ul> </li> </ul>
<b>Construction of new piers, culverts, etc.</b>	<b>Erosion. Harmful to aquatic life.</b> Alters streamflow within channel. Erosion/sedimentation upstream and downstream of structures. Reduces quality and quantity of aquatic habitat.	<ul style="list-style-type: none"> <li>▶ <b>Channel Stabilization</b></li> <li>▶ <b>Construction Phasing</b></li> </ul>
<b>Increased Stormwater Velocity &amp; Volume</b>	<b>Erosion. Harmful to aquatic life.</b> Increased stormwater runoff velocity and volume causes stream channelization (i.e., straightening). Channelization increases surface water velocity and exacerbates erosion and sedimentation. Reduces quality and quantity of aquatic habitat.	<ul style="list-style-type: none"> <li>▶ <b>Water Quality Ponds<sup>2</sup></b></li> <li>▶ <b>Channel Stabilization</b></li> </ul>

2 Notes: <sup>1</sup> See Section 3.7.4.1 for a description of proposed mitigation measures.

3 <sup>2</sup> The generic term Water Quality Ponds refers to ponds that accommodate water quality capture volume, detention ponds, or retention ponds. The exact BMP will be determined during final design.

4 <sup>3</sup> Activities CDOT currently undertake at construction sites and are required by permit.

1 If stormwater is left untreated, the project alternatives would cause indirect impacts later in  
2 time or at some distance downstream of the project area. These indirect impacts include  
3 alterations to natural channel movement processes (i.e. meandering, channel incision) and the  
4 continual degradation of aquatic habitat.

5 For each build alternative and the No-Action Alternative, surface water quality impacts were  
6 determined by evaluating the total impervious surface area, estimating the total areas of  
7 roadway that will be treated by BMPs, by comparing projected traffic volumes, and applying  
8 the Driscoll model.

9 **Impervious Surfaces.** The total impervious surface area of each alternative was evaluated as  
10 a way to estimate water quality impacts in the absence of BMPs. In addition, the impervious  
11 surface area treated by BMPs was also used to estimate overall water quality impacts from  
12 each build alternative and the No-Action alternative. Generally, if roadway runoff is passed  
13 through a BMP, the post-BMP runoff will have better quality than untreated runoff. This was  
14 quantified by comparing the impervious surface area associated with an alternative to the  
15 percent of that area being treated, or passed through, a BMP. Therefore, an alternative with a  
16 higher percentage of treatment will have a lesser impact to the water quality in the project area  
17 when compared to levels of existing BMP treatment (see **Table 3.7-4**). Areas of proposed  
18 water quality treatment were estimated based on current and future MS4 areas, the presence  
19 of sensitive waters, and the available area for BMPs within the right-of-way.

20 **Table 3.7-4 Summary of Total and Treated Impervious Areas**

Alternative	Total Impervious Area (acres)	Area Treated (acres)	% of Area Treated <sup>1</sup> (acres)
Existing	1,212	29	2.4%
No-Action	1,257	141	11.2%
Package A	1,946	1,765	90.7%
Package B	2,001	2,509	125%
Preferred Alternative	1,982	2,009	101%

Notes: <sup>1</sup> The percent of area treated through BMPs can be greater than 100 percent because the size of the ponds and/or depth of ponds are bigger/deeper to account for unknown constraints that may be identified in final engineering.

21 **Driscoll Model.** The Driscoll Model (FHWA, 1990), an FHWA-developed method, was applied  
22 as part of the impacts evaluation for the highway components. The modeling approach  
23 described herein is consistent with FHWA guidance and is used as a screening tool to  
24 compare predicted pollutant mass loading for existing conditions and predicted mass loadings  
25 from project alternatives (No-Action, Package A, Package B, and Preferred Alternative) before  
26 the application of BMPs. The constituents analyzed in the Driscoll Model were selected based  
27 upon their relation to roadway runoff and/or their sensitivity in the regional study area.  
28 Stormwater runoff concentration data for the constituents analyzed using the Driscoll Model  
29 were obtained from the I-70 Mountain Corridor Tier 1 Draft Programmatic Environmental  
30 Impact Statement (I-70 PEIS) (CDOT, 2004c) because stormwater runoff data is not available  
31 for the project area. Petroleum data was not available from the I-70 PEIS.

32 The results of the Driscoll Model are presented in **Table 3.7-5** by watershed. **Figure 3.7-4**  
33 graphically presents the Driscoll Model results by watershed. **Figure 3.7-4** presents predicted  
34 dissolved copper loading by watershed, because copper is a common roadway heavy metal  
35 pollutant.

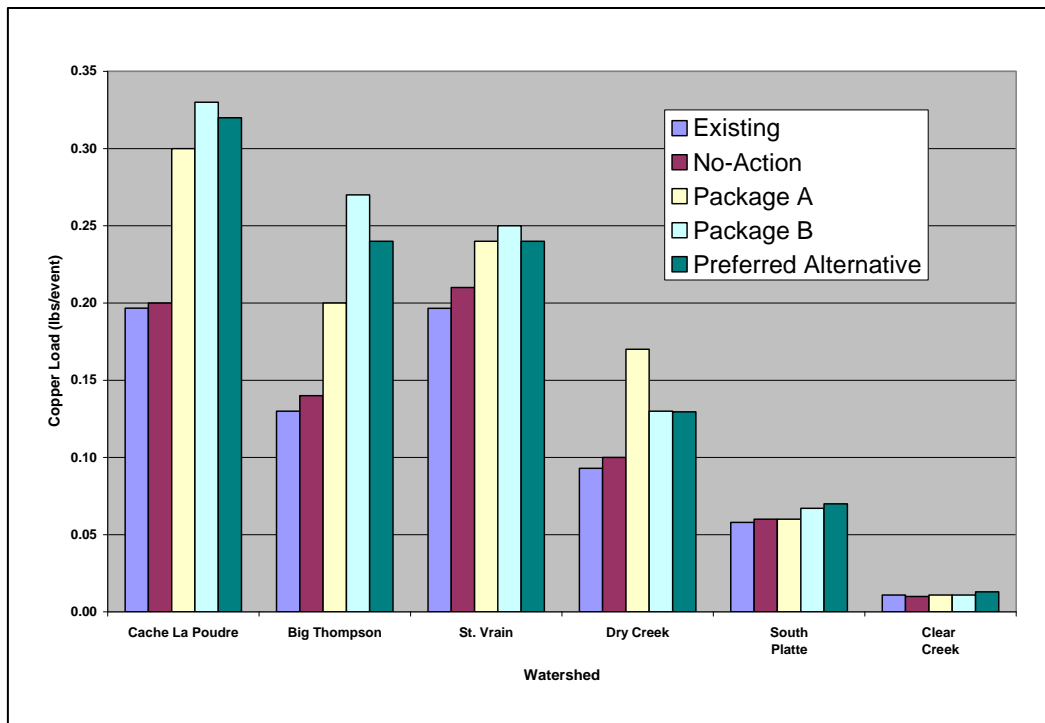
1 **Table 3.7-5 Driscoll Model Results by Watershed**

Contaminant	Alternative	Watershed						
		Cache La Poudre	Big Thompson	St. Vrain	Dry Creek	South Platte	Clear Creek	Total Loading
<b>Chloride (lbs/event)</b>	Existing	266	181	265	125	78.4	14.5	930
	No-Action	266	184	287	140	78.4	14.5	970
	Package A	400	272	323	229	81.1	14.5	1,320
	Package B	445	366	343	180	90.3	14.8	1,440
	Preferred Alternative	430	329	320	175	96.5	17.6	1,370
<b>Copper (lbs/event)</b>	Existing	0.20	0.13	0.20	0.09	0.06	0.011	0.69
	No-Action	0.20	0.14	0.21	0.10	0.06	0.010	0.72
	Package A	0.30	0.20	0.24	0.17	0.06	0.011	0.98
	Package B	0.33	0.27	0.25	0.13	0.07	0.011	1.06
	Preferred Alternative	0.32	0.24	0.24	0.13	0.07	0.013	1.01
<b>Phosphorus (lbs/event)</b>	Existing	12.4	8.4	12.4	5.8	3.7	0.7	43.4
	No-Action	12.4	8.6	13.4	6.5	3.7	0.7	45.3
	Package A	18.7	12.7	15.1	10.7	3.8	0.7	61.7
	Package B	20.8	17.1	16.0	8.4	4.2	0.7	67.2
	Preferred Alternative	19.9	15.2	14.8	8.1	4.5	0.8	63.4
<b>TSS (lbs/event)</b>	Existing	8,820	6,010	8,800	4,150	2,600	481	30,900
	No-Action	8,814	6,090	9,530	4,660	2,600	481	32,200
	Package A	13,300	9,040	10,700	7,610	2,690	481	43,800
	Package B	14,800	12,100	11,400	5,960	3,000	492	47,800
	Preferred Alternative	14,300	10,900	10,600	5,800	3,200	583	45,400
<b>Zinc (lbs/event)</b>	Existing	1.77	1.21	1.77	0.83	0.52	0.10	6.20
	No-Action	1.77	1.22	1.92	0.94	0.52	0.10	6.48
	Package A	2.66	1.82	2.15	1.53	0.54	0.10	8.78
	Package B	2.97	2.44	2.28	1.20	0.60	0.10	9.59
	Preferred Alternative	2.86	2.19	2.13	1.17	0.64	0.12	9.11

Notes: 1. Results presented in this table indicate modeled total pounds of contaminant discharged per component per event.  
2. Total loading values have been rounded to three significant figures.

2

1 **Figure 3.7-4 Driscoll Model Results by Watershed for Dissolved Copper**



2 Constituent loading is measured in pounds of constituent leaving the roadway per a median  
 3 rainfall event. The relationship between the alternative’s loading is the same for every  
 4 constituent analyzed in the Driscoll Model, only the magnitude of the loading changes. The  
 5 loads for the existing conditions are used as a “baseline” comparison for each build package.  
 6 The No-Action Alternative has the lowest predicted constituent loading of all of the project  
 7 alternatives.

8 Since the Driscoll Model is a screening tool that differentiates impacts among alternatives, the  
 9 results should not be used to determine if water quality standards are expected to be  
 10 exceeded. The loading information from the Driscoll Model is used to comparatively estimate  
 11 which alternative may have more water quality impacts. It can be assumed that an alternative  
 12 with a higher predicted load (i.e., a greater quantity of constituent leaving the road) would have  
 13 more water quality impacts than another alternative. Alternative-specific discussion of the  
 14 Driscoll Model results are presented in the following sections.

15 **Traffic.** Water quality impacts were also assessed by comparing the projected annual average  
 16 daily traffic (AADT) volumes. Several research studies have suggested that a correlation exists  
 17 between stormwater runoff quality and annual average daily traffic (AADT) volumes  
 18 (FHWA, 1990; Kayhanian and others, 2003). In general, urban areas with greater than  
 19 30,000 AADT have been shown to have higher pollutant concentrations of certain constituents  
 20 when compared with non-urban areas with AADT less than 30,000. However, the correlation  
 21 between AADT and pollutant concentrations is not consistent for all pollutants found in  
 22 highway runoff. Pollutants related to transportation activities, such as zinc and copper, are  
 23 expected to increase with AADT, while certain pollutants, such as total suspended solids, total  
 24 dissolved solids, and ammonia, which are commonly found in highway runoff but generally  
 25 associated with a non-urban setting are not expected to increase with AADT (FHWA, 1990;

Kayhanian and others, 2003). Therefore, if left unmitigated, it can be assumed that an alternative with an AADT greater than 30,000 would have higher concentrations of certain constituents in runoff than an alternative with an AADT less than 30,000.

**Table 3.7-6** presents the projected traffic volumes for the alternative components on I-25. The majority of the existing traffic volumes and all of the proposed traffic volumes are greater than 30,000 AADT. However, traffic volumes can still be used to compare alternatives from a water quality perspective. For example, an alternative with a higher traffic volume would be expected to have a higher amount of pollutants from vehicles being washed from the roadway; however the magnitude of difference is not enough to provide a noticeable difference in the negative effect to water quality. In general, the projected traffic volumes are relatively similar between the project alternatives and range from nearly two to three times the existing traffic volumes. The greatest travel demand is generated in the southern portion of the project area between E-470 to US 36 followed by SH 60 to E-470, SH 14 to SH 60, and SH 1 to SH 14.

**Table 3.7-6 Projected Traffic Volumes (AADT) from the North I-25 Project Alternatives**

Package	SH 1 to SH 14	SH 14 to SH 60	SH 60 to E-470	E-470 to 84th Ave
Existing	19,100 – 40,800	40,800 – 65,100	65,000 – 96,700	87,200 – 180,700
No-Action	31,600 - 72,300	72,300 - 127,400	116,800 - 188,000	167,500 - 246,400
Package A	37,600 - 93,000	93,000 - 160,600	128,000-202,900	171,400 - 248,200
Package B	37,600 - 92,000	92,000 - 149,100	115,000 - 200,300	183,700 – 253,500
Preferred Alternative	37,600 - 97,600	97,600 - 168,000	130,300 - 196,900	183,700 - 253,500

**Construction and Drainage.** Water quality impacts from construction activities are discussed qualitatively based upon the current state of practice for construction within CDOT. Impacts to the drainage system are briefly discussed in this section; however, the detailed analysis of the drainage system is presented in the **Section 3.9 Floodplains**.

### *Groundwater*

Groundwater quality impacts were evaluated by estimating the number of groundwater wells within the proposed right-of-way (see **Table 3.7-7**). This was evaluated because active groundwater wells would need to be relocated, and existing wells would need to be plugged, sealed, and abandoned. For wells located within the proposed right-of-way, the status and exact location of groundwater well use will have to be determined prior to construction activities to identify the necessary course of action. For example, active wells would require relocation, while inactive wells can be abandoned.



1 **Table 3.7-7 Summary of Groundwater Wells within the Project Area**

	Package A	Package B	Preferred Alternative
Potentially Impacted Groundwater Wells <sup>1</sup>	105	111	112

Note: <sup>1</sup> Includes all transit stations and associated parking lots and CDOT maintenance facilities and associated parking lots.

2 **3.7.3.2 NO-ACTION ALTERNATIVE**

3 The No-Action Alternative includes safety and maintenance improvements that would need to  
4 be constructed if the build packages were not implemented. Major and minor structure  
5 maintenance activities are expected to occur on I-25 from US 36 to SH 1. Safety  
6 improvements are anticipated at selected locations from WCR 34 to SH 1. See **Chapter 2**  
7 *Alternatives* for additional description of the No-Action Alternative. The No-Action Alternative  
8 does not include transit components.

9 *Surface Water*

10 **Impervious Surfaces.** Direct effects on surface water quality from increases in impervious  
11 surface area would be negligible under the No-Action Alternative. This is because the  
12 No-Action Alternative has relatively minor contributions of impervious surface area from any  
13 structure upgrades, such as interchange improvements or bridge replacements.

14 The quality of stormwater runoff would be dependent on the implementation of BMPs  
15 associated with No-Action Alternative activities within MS4 areas. Projects over one acre in  
16 size associated with the No-Action Alternative that are located within MS4 areas will require  
17 BMPs, thereby reducing impacts from increased impervious surface area. The percentage of  
18 the impervious surface area treated by BMPs for the No-Action Alternative is substantially less  
19 than either of the package alternatives. This means that the majority of stormwater runoff from  
20 I-25 would continue to not be treated prior to discharging to water bodies.

21 Under the No-Action Alternative, only 11.2 percent of the impervious surfaces within the  
22 project area are currently being treated. This area is within the SH 60 to E-470 component and  
23 the majority of increased pollutants deposited from vehicles would not pass through a BMP  
24 prior to discharge to receiving water bodies.

25 **Driscoll Model.** As previously mentioned, the results of the Driscoll Model are presented as a  
26 screening tool to differentiate impacts among alternatives and not to determine if water quality  
27 standards are expected to be exceeded. The No-Action Alternative has the lowest estimated  
28 contaminant loading of the four alternatives (see **Table 3.7-5**). The only watershed with an  
29 increase in loading greater than the existing conditions is the Clear Creek watershed. This  
30 component crosses the Big Thompson River, St. Vrain Creek, and Big Dry Creek watersheds.  
31 The remaining components have the same estimated loading as the existing conditions.

32 **Traffic.** While the amount of impervious surfaces for the No-Action Alternative is  
33 approximately 689 to 744 acres less than the build package alternatives, the increase in future  
34 traffic volumes should also be considered. Chemicals and other pollutants deposited along  
35 I-25 within the project area and mobilized within stormwater runoff would continue to increase  
36 as traffic volumes continue to increase along the I-25 highway corridor over time. The largest

1 potential increase in traffic would likely occur in the SH 60 to E-470 and E-470 to US 36  
2 components. This component crosses the Big Thompson River, St Vrain Creek, and Big Dry  
3 Creek watersheds.

4 **Construction.** Major and minor structure maintenance activities, such as demolition and  
5 construction of bridges and interchange improvements would have construction-related  
6 impacts at all stream crossings if left unmitigated. These impacts and the proposed mitigation  
7 to minimize these impacts are included in **Table 3.7-3**.

8 **Drainage System.** Major drainage impacts that result from cross drainage are addressed in  
9 **Section 3.9 Floodplains**. Minor drainage features includes storm drainage pipes, inlets, open  
10 channels, and other facilities that are used to convey local storm drainage.

11 Drainage improvements associated with the No-Action Alternative would occur in several  
12 areas where roadway improvements are currently planned. Anticipated drainage  
13 improvements for the No-Action Alternative would include a more efficient storm drainage  
14 system of pipes, inlets, open channels, and water quality facilities. There would be no drainage  
15 improvements for the E-470 to US 36 component in the No-Action Alternative and impacts  
16 from an inadequate drainage system would occur in this area. Approximately 11.2 percent of  
17 the total No-Action Alternative impervious surface is expected to be treated through a water  
18 quality BMP.

### 19 *Groundwater*

20 Groundwater impacts are not expected as a result of major and minor structure maintenance  
21 activities associated with the No-Action Alternative.

### 22 **3.7.3.3 PACKAGE A**

23 Package A contains both highway and transit components. The package includes construction  
24 of additional general purpose and auxiliary lanes on I-25 and implementation of commuter rail  
25 and commuter bus service. Construction of associated elements, such as commuter rail and bus  
26 stations, carpool lots, bridges, interchanges, and queue jumps, also was considered in this  
27 analysis. This package is described in detail in **Chapter 2 Alternatives**.

28 For purposes of this analysis, impervious surface areas include I-25 and associated  
29 interchanges, transit stations, maintenance facilities, and carpool lots. Rail lines were not  
30 included as impervious surfaces as part of this analysis because rail ballast material is relatively  
31 permeable.

### 32 *Surface Water*

33 **Impervious Surfaces.** Direct effects on surface water quality from all Package A components  
34 would result from the addition of paved impervious surfaces, primarily from highway widening  
35 for additional general purpose lanes and associated interchanges, bridges, and carpool lots.  
36 Package A has more total impervious surface area (1,946 total impervious surface acres) than  
37 the existing impervious area (1,212 total impervious surface acres), and the No-Action  
38 Alternative (1,257 total impervious surface acres). At the watershed level, impacts to water  
39 quality due to the addition of impervious surface area are expected to be the greatest,  
40 compared to the No-Action Alternative, as a result of highway widening in the Dry Creek  
41 Watershed.

1 To fully understand the impacts from impervious surface area for an alternative, it is important  
2 to consider the greater area surrounding the project. There are approximately 159,223 acres of  
3 total impervious surface area that exists within the regional study area from commercial and  
4 residential developments and other infrastructure. This gives context to the total impervious  
5 surface of Package A in relation to its surroundings that the impervious surface area  
6 associated with Package A is a small fraction (1.2 percent) of the overall impervious areas in  
7 the regional study area.

8 **Driscoll Model.** As previously mentioned, the results of the Driscoll Model are presented as a  
9 screening tool to differentiate impacts among alternatives and not whether or not water quality  
10 standards are expected to be exceeded. The Package A estimated contaminant load for the  
11 northern and southern components (SH 1 to SH 14 and E-470 to US 36, respectively) are  
12 slightly greater than the existing conditions. The Cache la Poudre and Big Thompson  
13 watersheds have the highest increased load from existing conditions, both approximately a  
14 50 percent increase. These watersheds show the greatest increase in loading because they  
15 have a large increase in impervious surfaces. The Package A components estimated loadings  
16 are less than the Package B components.

17 **Traffic.** In general, the projected traffic volumes are relatively similar between the project  
18 alternatives and range from nearly two to three times the existing traffic volumes (see  
19 **Table 3.7-6**). Package A would cause an increase in the amount of pollutants being washed  
20 from the roadway due to increased traffic volumes. All of the proposed traffic volumes for the  
21 Package A components are greater than 35,000 AADT. The greatest predicted travel demand  
22 is generated in the southern portion of the project area between E-470 to US 36 followed by  
23 SH 60 to E-470, SH 14 to SH 60, and SH 1 to SH 14. However, the SH 1 to SH 14 component  
24 would be expected to have the most significant increase in pollutants because existing traffic in  
25 this segment is at times currently less than 30,000 AADT, which is generally characteristic of  
26 non-urban areas. Project activities in this segment would cause traffic to increase to levels  
27 characteristic of urban areas (i.e., greater than 30,000 AADT), which have higher pollutant  
28 concentrations of certain constituents when compared with non-urban areas with AADT less  
29 than 30,000 (see **Section 3.7.3.1**).

30 If stormwater is left unmitigated, consequences from increased impervious surfaces and traffic  
31 would include an increase in water velocities and volumes (see **Table 3.7-4**). However, the  
32 incorporation of BMPs into the design will remove a large amount of the chemicals and  
33 sediment that could be deposited within surface water bodies within the project area. Under  
34 Package A, water quality ponds will provide a volume sufficient to treat approximately  
35 1,765 acres (90.7 percent of the impervious surfaces within the project area. This is compared  
36 to the existing 2.4 percent of the impervious surfaces within the project area that are currently  
37 being treated. Consequently, it is anticipated that water quality conditions will improve with  
38 Package A when compared to the existing or the No-Action Alternative conditions.

39 **Construction.** The implementation of Package A would result in construction-related impacts  
40 at all stream/ditch/canal crossings if left unmitigated. Other water bodies that may not cross  
41 I-25, but are within the construction footprint (including staging areas) would also be affected.  
42 The majority of construction related impacts results from the demolition and/or construction of  
43 structures, rail lines, and highway lanes. Construction-related impacts and the proposed  
44 mitigation to minimize these impacts are included in **Table 3.7-3**. The proposed construction  
45 mitigation measures are summarized in **Section 3.7.4** and are required by permit and policy  
46 on CDOT projects.

1 **Drainage.** Major drainage impacts that result from cross drainage are addressed in  
2 **Section 3.9 Floodplains.** General purpose lanes on I-25 for the SH 14 to SH 60 component  
3 and for the SH 60 to E-470 component would require that modifications be made to existing  
4 drainage systems or that a new drainage conveyance system be installed. By installing new  
5 drainage structures (e.g., storm drainage pipes, inlets, open channels and other facilities  
6 conveying local storm drainage), no additional impacts to the drainage system are anticipated.  
7 These structures could actually improve the drainage system when compared to the current  
8 and No-Action Alternative conditions.

### 9 *Groundwater*

10 The construction of Package A could require addressing up to 105 wells that are within the  
11 proposed right-of-way (see **Table 3.7-7**). The status and exact location of groundwater well  
12 use will have to be determined prior to construction activities to identify the necessary course  
13 of action for each well. Active wells would need to be relocated, and all active and non-active  
14 wells would need to be plugged, sealed, and abandoned.

### 15 **3.7.3.4 PACKAGE B**

16 Package B contains both highway and transit components. The package generally includes  
17 the construction of tolled express lanes on I-25 and implementation of bus rapid transit service.  
18 Construction of associated elements, such as bus stations, carpool lots, bridges, interchanges,  
19 and queue jumps, was also considered in the component-level analysis. This package is  
20 described in detail in **Chapter 2 Alternatives**.

21 For purposes of this analysis, impervious surface areas include I-25 and associated  
22 interchanges, transit station, maintenance facilities, and carpool lots.

### 23 *Surface Water*

24 **Impervious Surfaces.** Direct effects on surface water quality from Package B components  
25 would result from the addition of paved impervious surfaces, primarily from highway widening  
26 for additional tolled express lanes and associated interchanges, bridges, and carpool lots.  
27 Package B would result in more impervious surface area (2,001 acres) than the existing  
28 impervious area (1,212 acres), and the No-Action Alternative (1,257 acres). At the component  
29 level, impacts to water quality due to the addition of impervious surface area are expected to  
30 be the greatest in the Cache la Poudre River and Big Thompson River watersheds.

31 To fully understanding the impacts from impervious surface area for an alternative, it is  
32 important to consider the greater area surrounding the project. There are approximately  
33 159,223 acres of total impervious surface area that exist within the regional study area from  
34 commercial and residential developments and other infrastructure. This gives context to the  
35 total impervious surface of Package B in relation to its surroundings.

36 **Driscoll Model.** As previously mentioned, the results of the Driscoll Model are presented as a  
37 screening tool to differentiate impacts among alternatives and not whether or not water quality  
38 standards are expected to be exceeded. The Package B estimated contaminant load for the  
39 southern watersheds (Dry Creek, South Platte, and Clear Creek) are slightly greater than the  
40 existing conditions. The estimated loadings from the northern watersheds (Cache la Poudre  
41 and Big Thompson) are considerably greater than the existing conditions. The Cache la  
42 Poudre River and Big Thompson River watersheds have the highest increased load from  
43 existing conditions, approximately a 68 and 102 percent increase, respectively.

1 Package B has the greatest estimated loadings of all alternatives.

2 **Traffic.** In general, the projected traffic volumes are relatively similar between the project  
3 alternatives and range from nearly two to three times the existing traffic volumes (see  
4 **Table 3.7-6**). Package B would cause an increase in the amount of pollutants being washed  
5 from the roadway due to increased traffic volumes. All of the proposed traffic volumes for the  
6 Package B components are greater than 35,000 AADT. The greatest predicted travel demand  
7 is generated in the southern portion of the project area between E-470 to US 36 followed by  
8 SH 60 to E-470, SH 14 to SH 60, and SH 1 to SH 14.

9 If stormwater is left unmitigated, consequences from increased impervious surfaces and traffic  
10 would include an increase in water velocities and volumes, and an increase in the type and  
11 quantity of chemicals and other pollutants, such as sediment, that are deposited within the  
12 project area (see **Table 3.7-3**). However, the incorporation of BMPs into the roadway design  
13 will remove a large amount of chemicals and sediment deposited within surface water bodies  
14 within the project area. Under Package B, water quality ponds will provide a volume sufficient  
15 to treat approximately 2,509 acres (125 percent) of the impervious surfaces within the project  
16 area. This is compared to the existing 2.4 percent of the impervious surfaces within the project  
17 area that are currently being treated. Consequently, it is anticipated that water quality  
18 conditions will improve when compared to the existing and No-Action Alternative conditions.

19 **Construction.** The implementation of Package B would result in construction-related impacts  
20 at all stream/ditch/canal crossings if left unmitigated. Other water bodies that may not cross  
21 I-25, but are within the construction footprint (including staging areas) would also be affected.  
22 The majority of construction related impacts results from the demolition and/or construction of  
23 structures and highway lanes. Construction-related impacts and the typical mitigation to  
24 minimize these impacts are included in **Table 3.7-3**. The proposed construction mitigation  
25 measures are summarized in **Section 3.7.4**.

26 **Drainage.** Major drainage impacts that result from cross drainage are addressed in  
27 **Section 3.9 Floodplains**. The roadway improvements associated with Package B would  
28 require existing drainage system modifications or a new drainage conveyance system. By  
29 installing new drainage structures (e.g., storm drainage pipes, inlets, open channels and other  
30 facilities conveying local storm drainage), no additional impacts to the drainage system are  
31 anticipated. These structures could actually improve the drainage system when compared to  
32 the No-Action Alternative.

### 33 *Groundwater*

34 The construction of Package B could require the relocation of up to 111 wells that are within  
35 the proposed right-of-way (see **Table 3.7-7**). The status and exact location of groundwater well  
36 use will have to be determined prior to construction activities to identify the necessary course  
37 of action. Active wells would need to be relocated, and all active and non-active wells would  
38 need to be plugged, sealed, and abandoned.

39

### 3.7.3.5 PREFERRED ALTERNATIVE

The Preferred Alternative contains both highway and transit components. The package includes construction of additional general purpose and tolled express lanes on I-25 and implementation of commuter rail and commuter bus service. Construction of associated elements, such as commuter rail and bus stations, carpool lots, bridges, interchanges, and queue jumps, also was considered in this analysis. This package is described in detail in **Chapter 2** Alternatives.

For purposes of this analysis, impervious surface areas include I-25 and associated interchanges, transit stations, maintenance facilities, and carpool lots. Rail lines were not included as impervious surfaces as part of this analysis because rail ballast material is relatively permeable.

#### *Surface Water*

**Impervious Surfaces.** Direct effects on surface water quality from Preferred Alternative components would result from the addition of paved impervious surfaces, primarily from highway widening for additional general purpose lanes and associated interchanges, bridges, and carpool lots. The Preferred Alternative would result in more impervious surface area (1,982 acres) than the existing impervious area (1,212 acres), the No-Action Alternative (1,257 acres), and Package A (1,946 acres), but less than Package B (2,001 acres).

To fully understand the impacts from impervious surface area for an alternative, it is important to consider the greater area surrounding the project. There are approximately 159,223 acres of total impervious surface area that exists within the regional study area from commercial and residential developments and other infrastructure. This gives context that the total impervious surface of the Preferred Alternative is a small fraction (1.2 percent) of the overall impervious area in the regional study area. The greatest increase in impervious surface occurs in the Cache la Poudre and Big Thompson watersheds.

**Driscoll Model.** As previously mentioned, the results of the Driscoll Model are presented as a screening tool to differentiate impacts among alternatives and not to determine whether water quality standards are expected to be exceeded. The Preferred Alternative estimated contaminant load for the northern and central watersheds (Cache La Poudre, Big Thompson, St. Vrain, and Dry Creek, respectively) are slightly greater than the existing conditions. The estimated loadings from the two southern watersheds (South Platte and Clear Creek) are similar to the existing conditions and both Package A and Package B. The Cache la Poudre and Big Thompson watersheds have the highest increased load from existing conditions, both approximately a 50 percent increase. The Preferred Alternative components estimated loadings are generally higher than Package A components and less than the Package B components.

**Traffic.** In general, the projected traffic volumes are relatively similar between the project alternatives and range from nearly two to three times the existing traffic volumes (see **Table 3.7-6**). The Preferred Alternative would cause an increase in the amount of pollutants being washed from the roadway due to increased traffic volumes. All of the proposed traffic volumes for the Preferred Alternative components are greater than 35,000 AADT. The greatest predicted travel demand is generated in the southern portion of the project area between E-470 to US 36, followed by SH 60 to E-470, SH 14 to SH 60, and SH 1 to SH 14. However, the SH 1 to SH 14 component would be expected to have the most significant increase in

1 pollutants because existing traffic in this segment is at times currently less than 30,000 AADT,  
2 which is generally characteristic of non-urban areas. Project activities in this segment would  
3 cause traffic to increase to levels characteristic of urban areas (i.e., greater than  
4 30,000 AADT), which have higher pollutant concentrations of certain constituents when  
5 compared with non-urban areas with AADT less than 30,000 (see **Section 3.7.3.1**).

6 If stormwater is left unmitigated, consequences from increased impervious surfaces and traffic  
7 would include an increase in water velocities and volumes (see **Table 3.7-4**). However, the  
8 incorporation of BMPs into the design will remove a substantial amount of the chemicals and  
9 sediment that could be deposited within surface water bodies within the project area. Under  
10 the Preferred Alternative, water quality ponds will provide a volume sufficient to treat  
11 approximately 2,009 acres (101 percent) of the impervious surfaces within the project area.  
12 This is compared to the existing 2.4 percent of the impervious surfaces within the project area  
13 that are currently being treated. Consequently, it is anticipated that water quality conditions will  
14 improve with the Preferred Alternative when compared to the existing or the No-Action  
15 Alternative conditions.

16 **Construction.** The implementation of the Preferred Alternative would result in construction-  
17 related impacts at all stream/ditch/canal crossings if left unmitigated. Other water bodies that  
18 may not cross I-25, but are within the construction footprint (including staging areas) would  
19 also be affected. The majority of construction related impacts results from the demolition  
20 and/or construction of structures, rail lines, and highway lanes. Construction-related impacts  
21 and the proposed mitigation to minimize these impacts are included in **Table 3.7-3**. The  
22 proposed construction mitigation measures are summarized in **Section 3.7.4** and are required  
23 by permit and policy on CDOT projects.

24 **Drainage.** Major drainage impacts that result from cross drainage are addressed in  
25 **Section 3.9 Floodplains**. General purpose lanes on I-25 for the SH 14 to SH 60 component  
26 and for the SH 60 to E-470 component would require that modifications be made to existing  
27 drainage systems or that a new drainage conveyance system be installed. By installing new  
28 drainage structures (e.g., storm drainage pipes, inlets, open channels and other facilities  
29 conveying local storm drainage), no additional impacts to the drainage system are anticipated.  
30 These structures could actually improve the drainage system when compared to the current  
31 and No-Action Alternative conditions.

### 32 *Groundwater*

33 The construction of the Preferred Alternative could require addressing up to 112 wells that are  
34 within the proposed right-of-way (see **Table 3.7-7**). The status and exact location of  
35 groundwater well use will have to be determined prior to construction activities to identify the  
36 necessary course of action for each well. Active wells would need to be relocated, and all  
37 active and non-active wells would need to be plugged, sealed, and abandoned.

## 38 **3.7.4 Mitigation Measures**

39 This section summarizes the BMPs that have been incorporated as water quality mitigation  
40 measures into the build packages.

41

### 3.7.4.1 SURFACE WATER QUALITY

If stormwater runoff is left unmitigated, the No-Action Alternative, Package A, Package B, and the Preferred Alternative would have water quality impacts due to changes in stormwater characteristics from the addition of impervious surface area and increases in traffic levels. Other impacts would result from the demolition and construction of roadways and structures (e.g., bridges, culverts, piers, retaining walls) near surface water bodies. To reduce the impacts to water resources, a combination of mitigation measures consisting of permanent structural, nonstructural, and temporary construction BMPs will be implemented in the project area, in compliance with the Clean Water Act and CDOT's MS4 permit requirements. BMPs will include water collection and passive treatment of stormwater, which is currently being directly discharged into existing water systems. In addition, the BMPs may also provide protection to receiving waters from chemical spills that could occur in the project area. During the design process, CDOT will coordinate and share information with other MS4 Permit holders to ensure that the project is in compliance with all applicable permits. If there is an identified conflict in requirements, the most stringent regulations or specifications will be followed.

#### *Structural BMPs*

Permanent structural BMPs have already been identified and sited for major stream systems in the project area. Permanent structural BMPs will be constructed with the project and maintained to ensure their functionality. Water quality ponds and riprap outlet protection are examples of structural BMPs. The performance criteria for the water quality ponds within the project area are consistent with CDOT's current MS4 design criteria identified in the New Development and Redevelopment Program (CDOT, 2004b). During the design phase, the ponds or other structural BMPs will comply with the current requirement in the New Development and Redevelopment Program. The removal efficiencies for these types of BMPs (e.g., extended detention basin) are 50 percent to 70 percent (TSS), 10 percent to 20 percent (total phosphorus), and 30 percent to 60 percent (total zinc) (CDOT, 2004b).

**Water Quality Ponds.** Extended detention/retention ponds have been identified as the primary structural BMP for this project. Another example of a structural BMPs is an underground vault. The exact type of structural BMP will be determined during final design.

The No-Action Alternative has only two areas with BMPs (water quality ponds), associated with the No-Action Alternative improvements. Additional water quality ponds have been incorporated into the design of Package A, Package B, and the Preferred Alternative. Physical design constraints, adjacent property uses, and right-of-way requirements were analyzed and considered during the design process. It is anticipated that types and sizes of BMPs could be modified in the future. When possible, passive BMPs (e.g., grass swales or natural infiltration) will be used for ephemeral streams along the corridor that could reasonably discharge pollutants into perennial stream systems. An option for future consideration is to investigate the ability to develop a regional water quality plan between multiple MS4s and watershed organizations. The preliminary drainage design for Package A, Package B, and the Preferred Alternative is based on the CDOT Drainage Design Manual (CDOT, 2004a) and Volume 3 of the Urban Drainage and Flood Control District (UDFCD) Urban Storm Drainage Criteria Manual (UDFCD, 2010).



1 As previously mentioned, during final design, BMPs other than water quality ponds may be  
2 used. It is anticipated that any alternate BMPs will be able to be incorporated within the right-  
3 of-way identified for this EIS.

4 The locations for water quality ponds have been identified throughout the project area for  
5 Package A, Package B, and the Preferred Alternative. The placement of these BMPs was  
6 determined using a rating system that was based on existing and likely future MS4 areas,  
7 locations of sensitive surface water systems and/or irrigation canals, and physical design  
8 opportunities. More detailed information on BMP placement is provided in the Water Quality  
9 and Floodplains Technical Report (FHU, 2008b) and Addendum (FHU, 2011b).

10 **Figures 3.7-6, 3.7-7, and 3.7-8** show the areas along the I-25 corridor where water quality  
11 ponds are proposed. They also show the reason why ponds were included in each particular  
12 stretch of the corridor. As previously discussed, Package A would provide ponds with a capacity  
13 to treat 90.7 percent of the total impervious surface area, while Package B would provide ponds  
14 with a capacity to treat 125 percent of the total impervious surface area. The Preferred  
15 Alternative would provide ponds with a capacity to treat 101 percent of the total impervious area.  
16 A percentage greater than 100 indicates that the volume provided is greater than the defined  
17 water quality capture volume, which is equal to one-half inch of rainfall times the impervious  
18 area. These are dramatically greater than the existing conditions (2.4 percent) and the No-Action  
19 Alternative (11.2 percent).

20 Water quality ponds are only proposed along the I-25 corridor. No roadway improvements are  
21 proposed along the US 85 corridor. The only additions of impervious surface are very small  
22 bus queue jumps at select intersections in Package A. The WQCV for these queue jumps is  
23 less than 0.1 acre-feet. These improvements are a great distance from the primary additional  
24 impervious surface areas (approximately 7 miles) and as such, permanent water quality  
25 features are not included at these locations. It is not practical to place water quality ponds  
26 along the US 85 corridor because a new drainage system would be required to carry the water  
27 to a BMP.

28 The application of water quality ponds as part of the Preferred Alternative is expected to  
29 reduce the amount of iron discharged from the roadway to Segment 1 of Big Dry Creek, which  
30 is on CDPHE's Monitoring and Evaluation list for Iron, by approximately 50 to 60 percent  
31 (FDEP, 1999). The improvements in this area, where Segment 1 of Big Dry Creek lies, are  
32 expected to increase all pollutant loadings—including iron—by approximately 30 percent (see  
33 **Table 3.7-10**). This demonstrates that the water quality ponds can improve the water quality  
34 conditions at Big Dry Creek over the existing conditions. The same reduction would be  
35 expected for Package B since the project footprint is the same in this segment of the  
36 watershed. However, Package A does not have any roadway improvements in this area and  
37 therefore no water quality ponds would be provided to reduce the current iron loadings from  
38 the No-Action Alternative conditions.

39 Dissolved copper removal in water quality ponds is less than that of iron. Dissolved copper in  
40 Package A, Package B, and the Preferred Alternative are estimated to increase by 42, 59, and  
41 47 percent, respectively, over the existing conditions. Data from the USEPA shows that  
42 dissolved copper in extended dry detention basins ranges from 1.4 to 38 percent removal  
43 (USEPA, 2008). While this is a wide range, it does show that there is potential for the  
44 proposed water quality ponds to remove dissolved copper to a level close to existing  
45 conditions.

1 As previously stated, removal efficiencies of 50 to 70 percent for Total Suspended Solids  
2 (TSS), 10 to 20 percent for total phosphorus, and 50 to 60 percent for iron are expected for the  
3 proposed water quality ponds.

4 **Riprap.** Riprap will be placed at bridge abutments, piers, and at critical portions of a channel  
5 or floodplain to avoid progressive or catastrophic failure of a structure. Riprap reduces water  
6 quality impacts by protecting stream systems from accelerated erosion and sedimentation  
7 processes that could occur from structures (see **Table 3.7-8**). The most effective method of  
8 stabilization at bridge abutments and piers is the use of riprap. Riprap that is correctly sized, is  
9 angular, and placed on a granular material or fabric, has a better record for erosion and scour  
10 protection than other methods such as vegetative cover. Despite its reliability, riprap must still  
11 be monitored and maintained. More detailed information on riprap layout and aesthetics is  
12 provided in the Water Quality and Floodplains Technical Report (FHU, 2008c) and Addendum  
13 (FHU, 2011b).

14 Energy dissipation devices or materials, such as riprap, will control post-construction erosion  
15 near the bridge. According to SB40 Guidelines, riprap used above the ordinary high water level  
16 of the river that is not directly under a bridge must be covered with topsoil and vegetated.

#### 17 *Nonstructural BMPs (Construction and Post-Construction)*

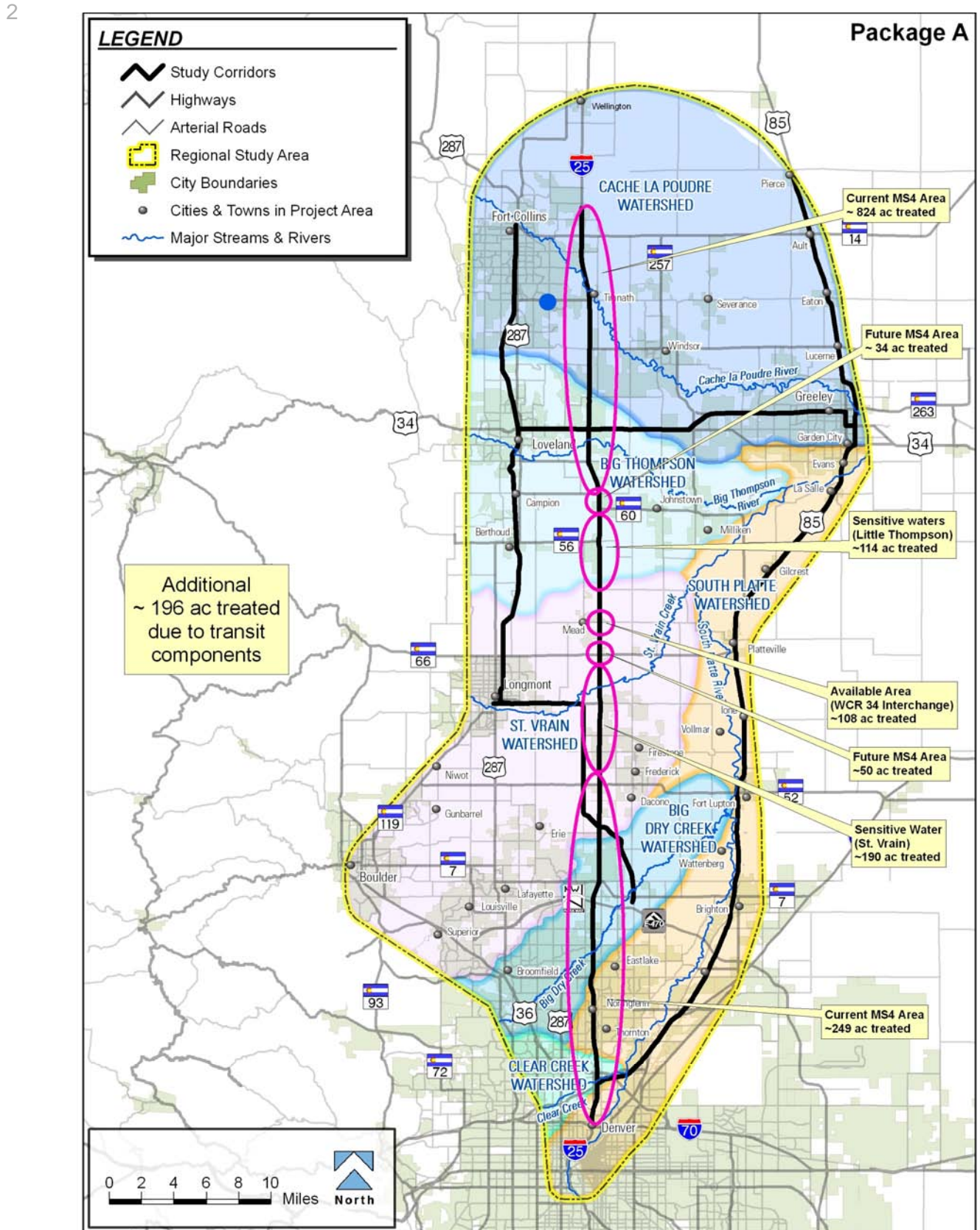
18 Nonstructural BMPs reduce or eliminate pollutant mobilization within stormwater runoff. Street  
19 sweeping, snow storage, and spill containment measures are examples of post-construction  
20 nonstructural BMPs. Project construction phasing is another nonstructural BMP to be  
21 implemented to minimize water quality impacts. Phasing construction activities minimizes the  
22 effects associated with large areas of exposed ground and with soil compaction from heavy  
23 machinery use, both of which are commonly associated with transportation projects.  
24 Construction nonstructural BMPs include mulch/mulch tackifier, vegetated buffer strips, and  
25 preservation of mature vegetation.

#### 26 *Temporary Structural BMPs (Construction)*

27 There is also potential for impacts to surface water bodies during the demolition and  
28 construction of roadways and structures (e.g., bridges, culverts, piers, retaining walls). A  
29 Stormwater Management Plan and Notebook will be prepared in accordance with the current  
30 CDOT practices to ensure that temporary construction impacts are avoided or minimized.  
31 Temporary structural BMPs are implemented to control erosion and sediment associated with  
32 areas of ground disturbance while construction activities take place. These measures remain  
33 in place until CDOT determines they are no longer needed at the construction sites, such as  
34 when soil stabilizing vegetation has been reestablished. Silt fences, straw bale barriers, and  
35 temporary check dams are examples of temporary structural BMPs used during construction.

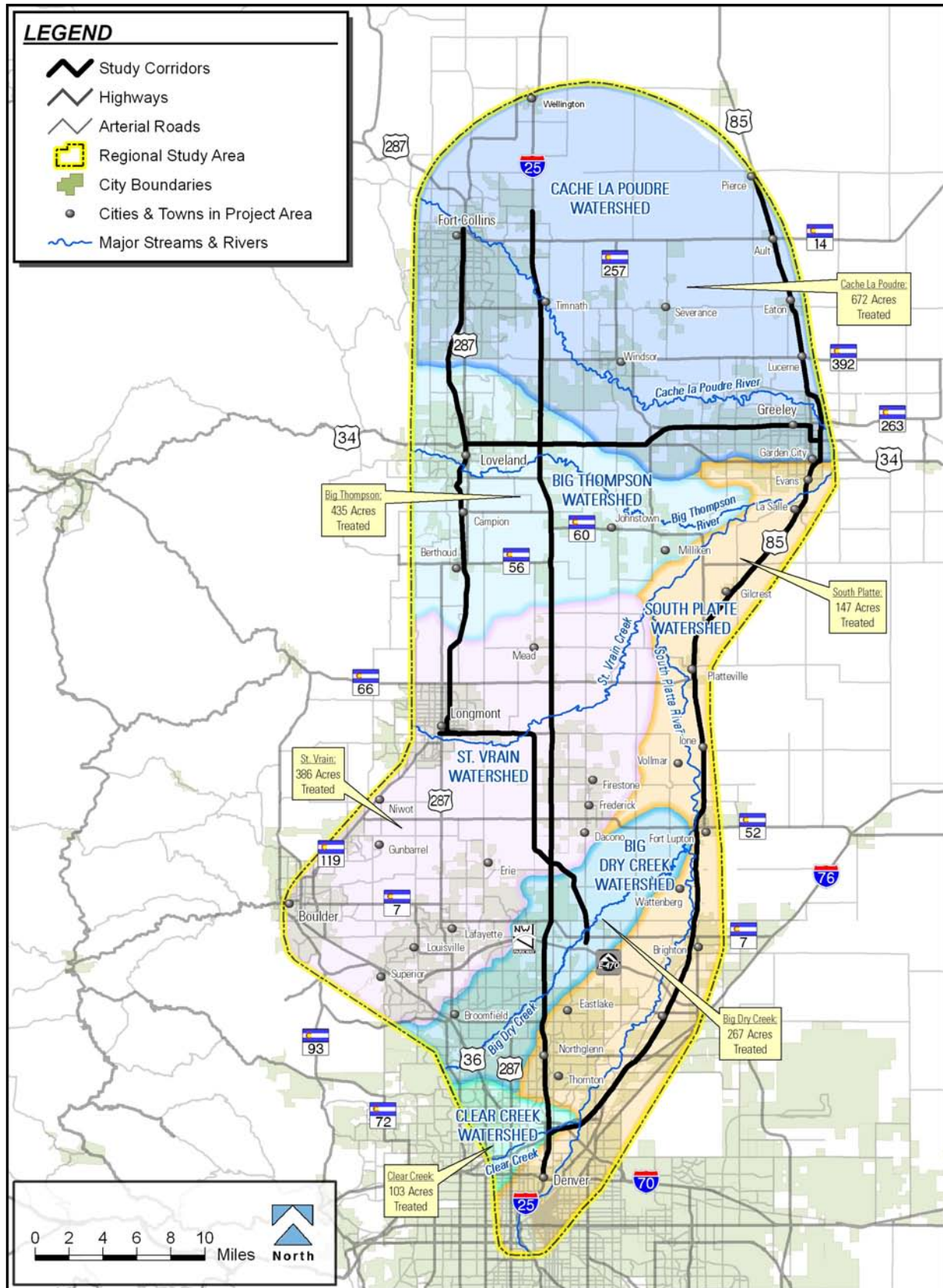
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1 Figure 3.7-5 Package A – Areas of Future Water Quality Treatments





1 Figure 3.7-7 Preferred Alternative - Areas of Future Water Quality Treatments



1 CDOT's specifications for managing stormwater at a construction site (currently specifications  
2 107.25, 208, 212, 213, and 216) will be followed. When put into practice, the actions identified  
3 below will help avoid construction impacts:

- 4 ▶ If lead paint is present, this material must not be allowed to flake off and enter receiving  
5 waters. (Section 402, Clean Water Act, CDPHE Regulation 61).
- 6 ▶ If cranes and other equipment are used for bridge demolition within a river or streambank  
7 area, the equipment will be kept out of the river as much as practicable, or per compliance  
8 with Section 404 permit, and all work shall minimize temporary impacts to the river. The  
9 creation of a crane pad is necessary if cranes or other equipment cannot be kept out of the  
10 river.
- 11 ▶ There is a potential for sediment to enter streams from land disruption and subsequent  
12 erosion. Therefore, BMPs such as protecting existing vegetation, placing structural BMPs,  
13 and limiting access areas will be implemented in compliance with the CDPHE general  
14 construction permit. Stormwater management plans must be developed during design and  
15 implemented during construction, and updated as needed to keep the project in  
16 compliance with the CDPS-SCP permit for the site.
- 17 ▶ Caissons used to create bridge piers could require groundwater dewatering. A discharge  
18 permit and a treatment strategy may be needed before dewatering activities can occur.
- 19 ▶ If other regulated materials are present within or on structures, they must be removed and  
20 appropriately recycled or disposed of prior to demolition activities. Typical materials include  
21 containerized regulated liquids such as paints, solvents, oil, grease, chemicals, pesticides,  
22 and herbicides, and chlorofluorocarbon (CFC) containing equipment (equipment must be  
23 emptied before equipment is removed) [Colorado Hazardous Waste Regulations (6 Colorado  
24 Code of Regulations [CCR] 1007-3)].
- 25 ▶ Senate Bill 40 (SB40) certification from the CDOW is required when construction occurs in  
26 "any streams or its banks or tributaries". This permit coordination will include identification  
27 of measures to protect existing riparian areas, such as mitigating stormwater runoff or  
28 replacing riparian vegetation (on a 1:1 basis for trees and a square footage basis for  
29 shrubs).

30 Permanent structural BMPs, nonstructural BMPs, and temporary construction BMPs must be  
31 regularly inspected and maintained to ensure functionality and efficiency. This includes  
32 inspections of proper BMP operation, outfall discharges and erosion protection, and detention  
33 pond sediment removal.

#### 34 **3.7.4.2 GROUNDWATER QUALITY**

35 The status of groundwater well use will have to be determined prior to construction activities to  
36 identify if active wells are present. Active wells in the final right-of-way will need to be relocated  
37 and non-active wells would need to be plugged, sealed, and abandoned.

38 All wells that lie within the proposed right-of-way will be included in all project specifications  
39 and plan drawings. If any of these wells are affected by project activities, coordination with the  
40 Colorado Department of Labor and Employment, Division of Oil and Public Safety will be  
41 required. If necessary, wells must be plugged, sealed, and abandoned according to CDOT  
42 Section 202.02 Standard Specifications for Road and Bridge Construction and in conformance  
43 with the State Engineer well abandonment procedures.

1 If groundwater is encountered during activities associated with excavations for  
2 caisson/retaining walls, the discharge of groundwater is authorized if the following conditions  
3 are met and then a dewatering permit is not required:

- 4 ▶ A Construction Stormwater Permit has been obtained;
- 5 ▶ the source is groundwater and/or groundwater combined with stormwater that does not  
6 contain pollutants in concentrations exceeding the State groundwater standards in  
7 Regulations 5 CCR 1002-41 and 42;
- 8 ▶ the discharge is in accordance with the CDPHE-WQCD Water Quality Policy-27, Low-Risk  
9 Discharges—September 2009;
- 10 ▶ the source is identified in the Stormwater Management Plan (SWMP);
- 11 ▶ dewatering BMPs are included in the SWMP, and
- 12 ▶ these discharges do not leave the site as surface runoff or to surface waters.

13 If these conditions are not met, then a separate Clean Water Act Section 402 Construction  
14 Dewatering Permit or Individual Construction Dewatering Permit will be required to be obtained  
15 from the CDPHE - WQCD. In addition, if dewatering is necessary, groundwater brought to the  
16 surface will be managed according to Section 107.25 of the CDOT *Standard Specifications for*  
17 *Road and Bridge Construction* (CDOT, 2005c).

### 18 3.7.4.3 DRAINAGE

19 Approximate locations of water quality ponds are shown in the *Concept Plans Technical*  
20 *Report* (FHU and Jacobs, 2011b). Higher flows will be allowed to pass off of the right-of-way  
21 and into a drainageway. Storm drainage should be separated from irrigation facilities,  
22 wetlands, and sensitive areas. Drainage at bridges, super elevation transitions, ramp gores,  
23 and low areas will be analyzed and coordinated into the design. This detailed storm drainage  
24 for the any build alternative will be determined during final design.

25 The implementation of commuter rail or bus service will require similar drainage  
26 improvements. The CDOT *Drainage Design Manual*, the CDOT *Erosion Control and*  
27 *Stormwater Quality Guide* (CDOT, 2002b), and the Urban Drainage and Flood Control District  
28 *Urban Storm Drainage Criteria Manual* will be consulted for guidance during design.

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